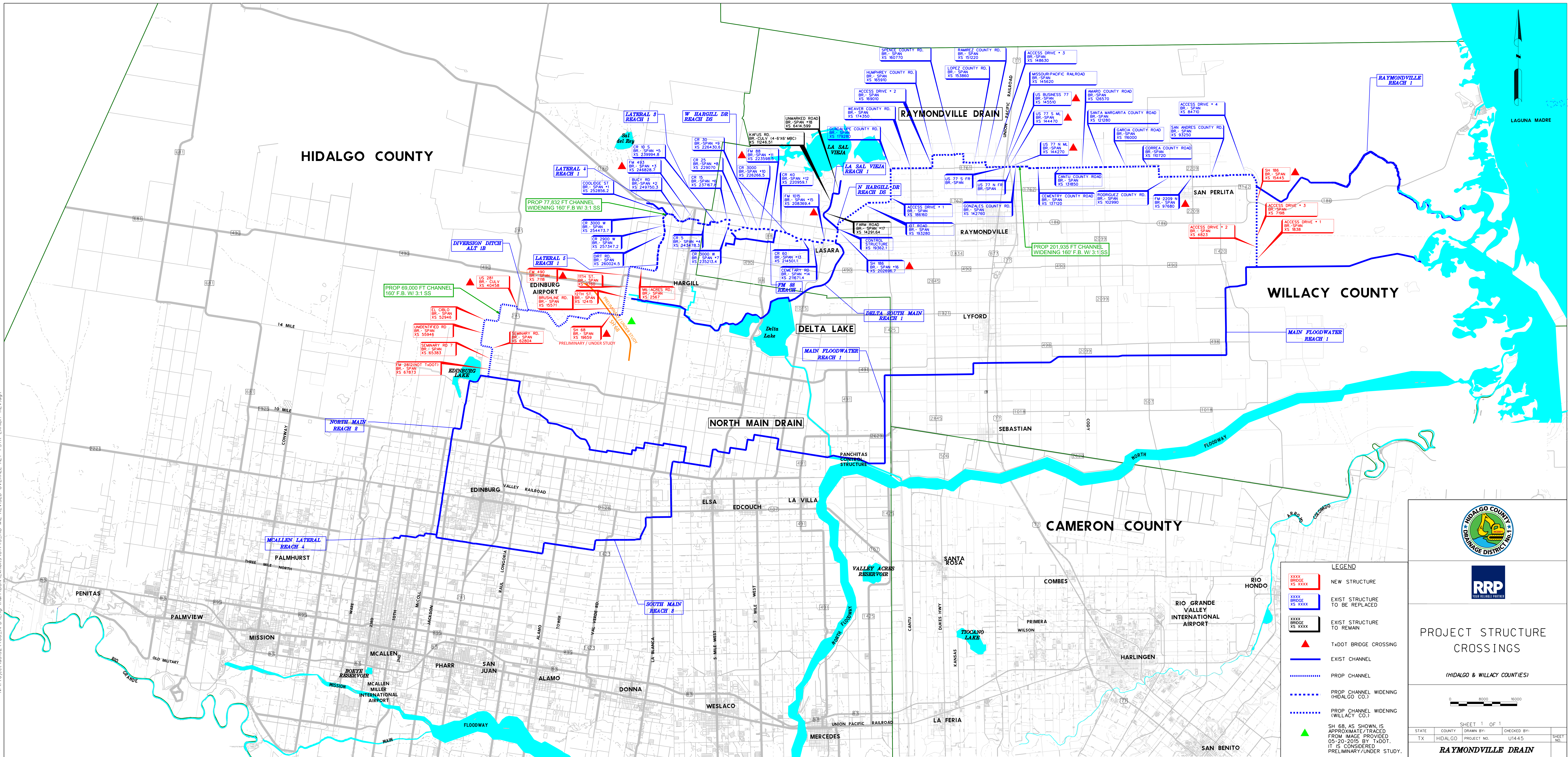
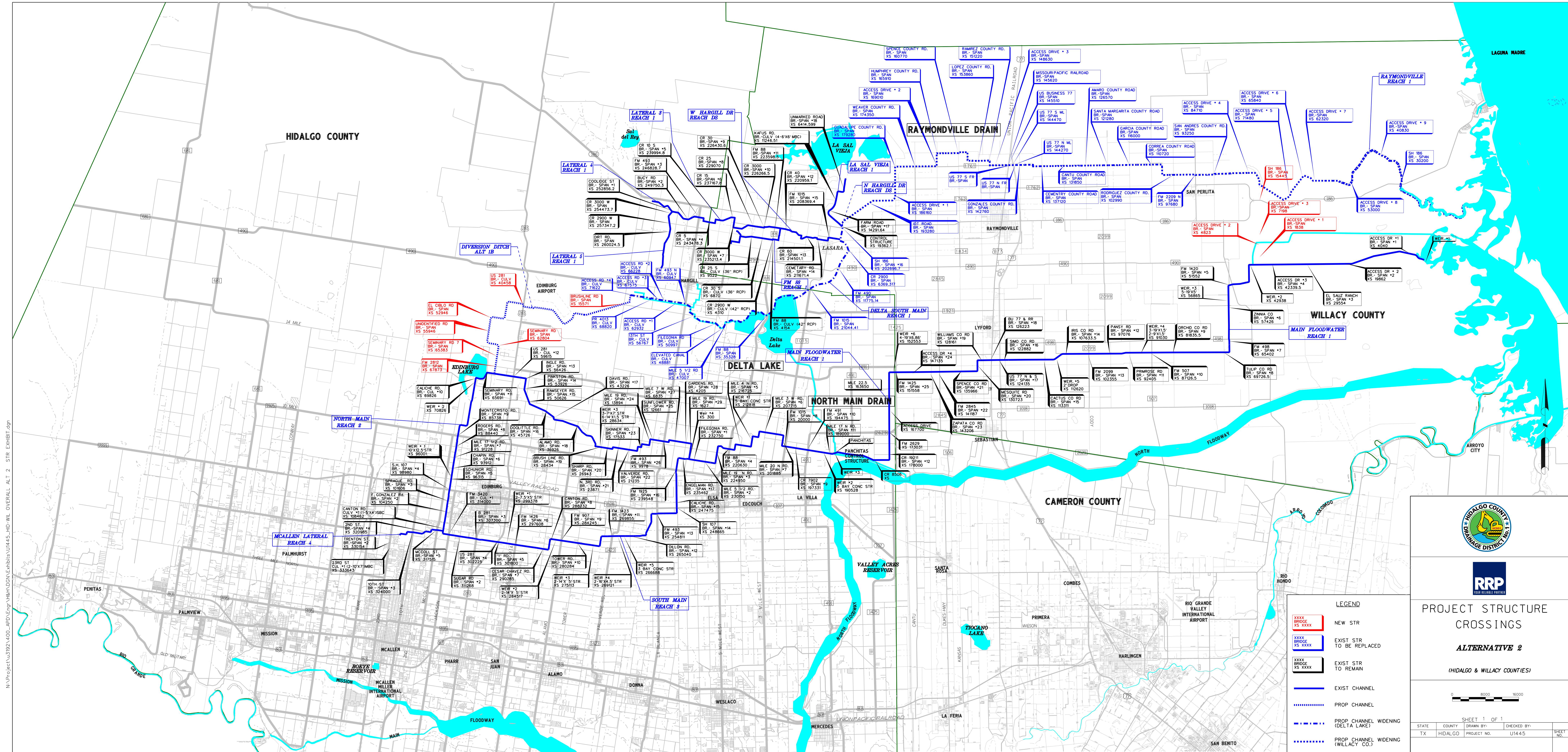

**RAYMONDVILLE DRAIN PROJECT
ENGINEERING APPENDIX A1**

ATTACHMENT B

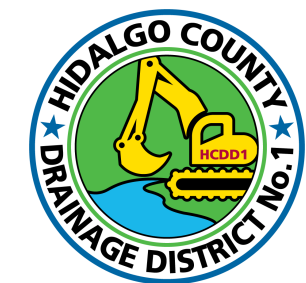
HYDRAULICS

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PROJECT STRUCTURE CROSSINGS

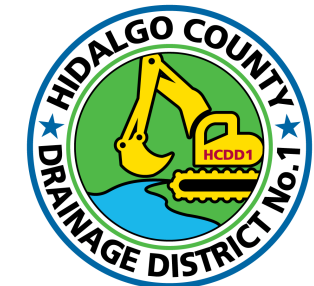
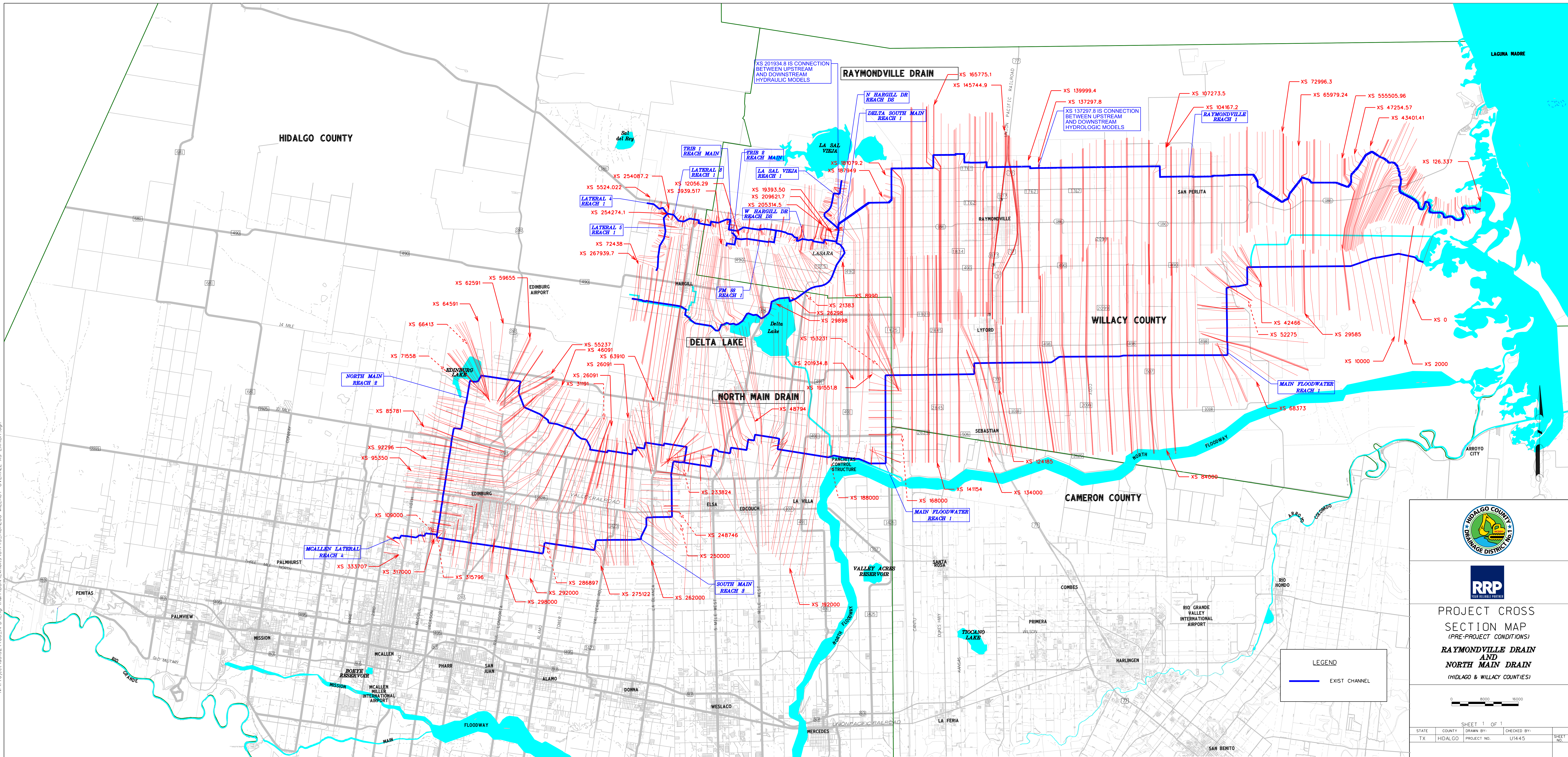
ALTERNATIVE 2

(HIDALGO & WILLACY COUNTIES)

0 8000 16000

SHEET 1 OF 1		DRAWN BY:		CHECKED BY:	
STATE	COUNTY	PROJECT NO.	U1445		
TX	HIDALGO				

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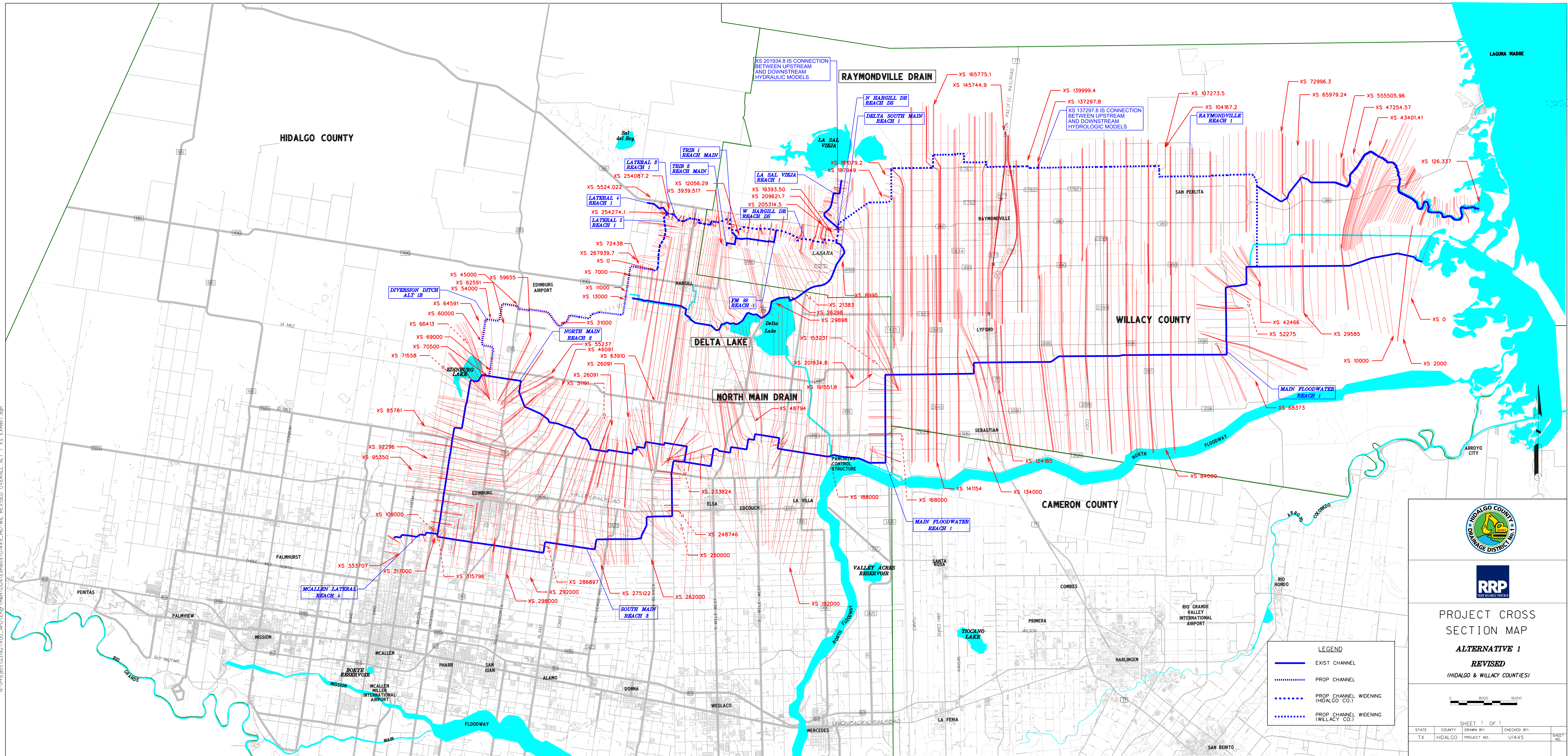


PROJECT CROSS
SECTION MAP
(PRE-PROJECT CONDITIONS)
**RAYMONDVILLE DRAIN
AND
NORTH MAIN DRAIN**
(HIDALGO & WILLACY COUNTIES)

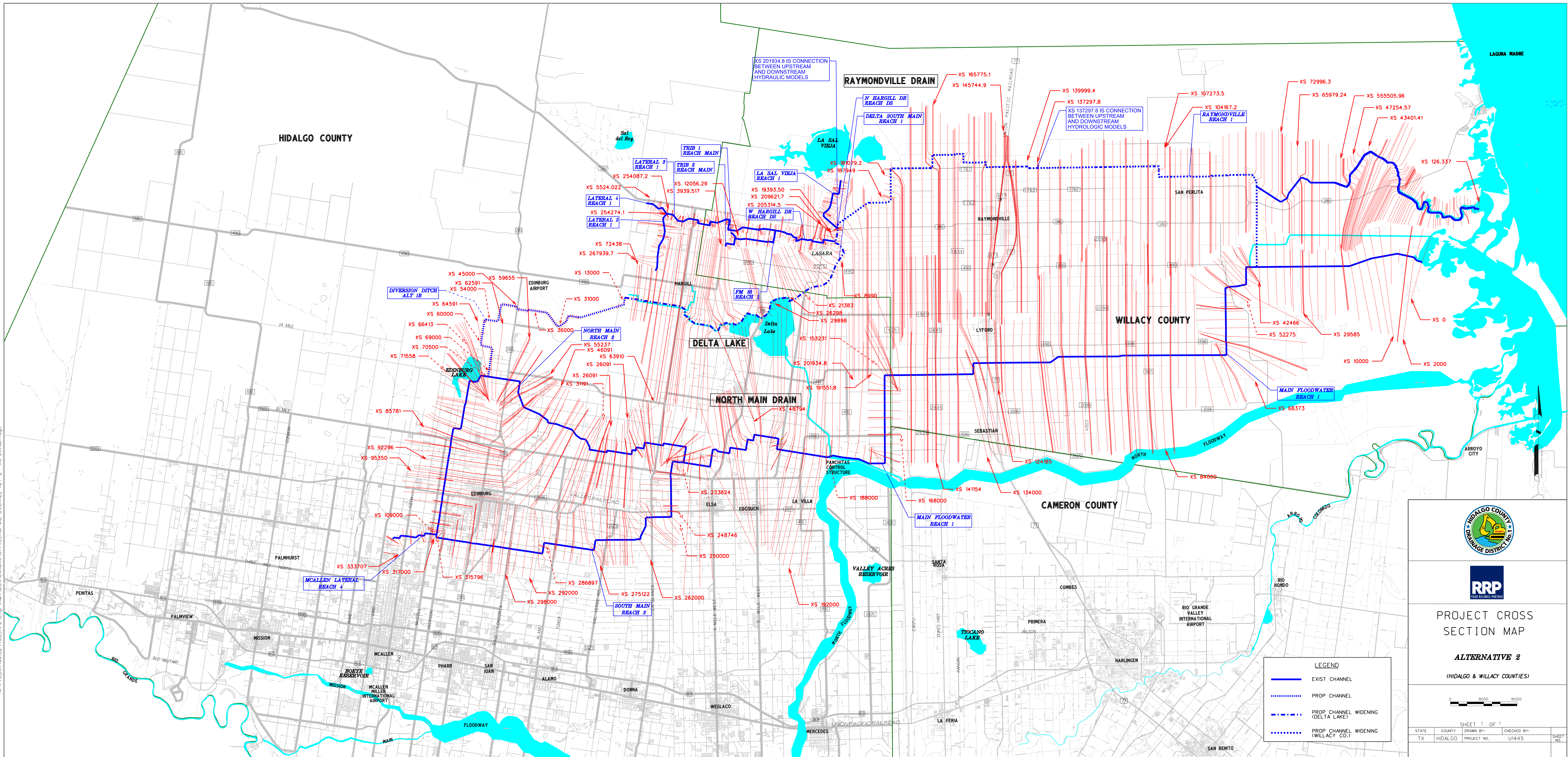
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LEGEND

- EXIST CHANNEL
- PROP CHANNEL
- PROP CHANNEL WIDENING (DELTA LAKE)
- PROP CHANNEL WIDENING (WILLACY CO.)



PROJECT CROSS SECTION MAP

ALTERNATIVE 2
(HIDALGO & WILLACY COUNTIES)



SHEET 1 OF 1		STATE	COUNTY	DRAWN BY:	CHECKED BY:	SHEET NO.
TX	HIDALGO	PROJECT NO.	U1445			

**Raymondville Drain
Design Task Protocol – 2
Hydraulics**

The Raymondville Drain is located in the Rio Grande Valley of South Texas including four towns; Raymondville, San Perlita, La Sara, and Hargill. The watershed flows in a west to east direction and the total area is approximately 355 square miles. The HEC-RAS model performed by RRP covers the main stems of the Raymondville drain beginning at a junction just south of the diversion to La Sal Vieja and proceeding upstream. Note that this junction is not at the same location as HEC-HMS. In response to RRP's submittal on January 2005, the USACE wrote a draft white paper titled "Raymondville Drain Pre-Project Conditions Report," April 2006. (USACE RDPR) RRP's model is referred to as the 'Upstream RAS Model' since USACE developed the Downstream RAS Model. RRP reevaluated the HEC-RAS model during September 2006 and determined required changes described below.

Required Changes to Upstream HMS Model

RRP performed the hydraulic model in HEC-RAS Version 4.1. The following revisions are needed to correct the model.

1. Raymondville HEC-RAS model levee points need to be set on channel banks to eliminate flow in overbank areas. When water surface overtops levee, flow will occur at that point in the overbanks. Currently cross sections are defining the levees as ineffective flow locations. This methodology is incorrect as it allows flow to enter the overbank areas and reduces flow in channel.
2. Flow revisions are required to both the Raymondville Drain and the North Main Drain to represent revised hydrology. In addition, new flow additions are needed. The RRP 2005 models have river reaches of over 3 miles with no flow increase or decrease.
3. The bridge modeling approach assumes the energy equation and does not account for momentum or Yarnell (Piers) in low flow calculations. Also, pressure and weir flow is not accounted for in high flow situations.
4. The downstream and upstream embankments are not included and left as zero in the culvert and bridge models. The bridges have no abutments modeled. Minimum weir flow elevations are not included.
5. Culverts at same location are modeled as No. 1, No. 2, etcetera rather than a multiple box or as a multiple opening.
6. Expansion and contraction coefficients should be 0.3 and 0.5 at upstream and downstream cross sections from bridges and culverts, not 0.1 and 0.3. This is prevalent at all crossings.
7. Manning's n values used universally on all stream was $LOB = 0.035$, $channel = 0.05$, and $ROB = 0.035$. This should be changed to $LOB = 0.1$, $Channel = 0.06$ to 0.1 , and $ROB=0.1$.

References

Feldman, Arlen D., "Hydrologic Modeling System HEC-HMS Technical Reference Manual,"
U.S. Army Corps of Engineers, Hydrologic Engineering Center, HEC, 609 Second St.,
Davis, CA 95616-4687, March 2000.

U.S. Army Corps of Engineers Galveston District, "Draft Raymondville Drain Pre-Project
Conditions Report," April 2006.